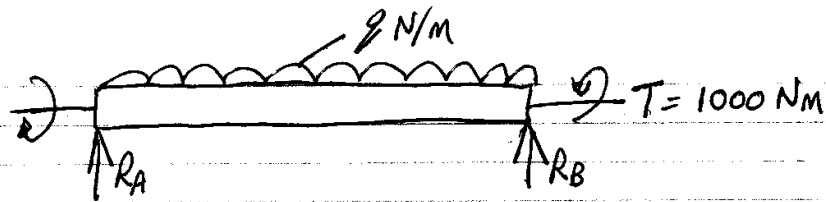


Q1.



$$L = 2\text{m}$$

$$d_o = 75\text{mm}$$

$$d_i = 60\text{mm}$$

$$A = \frac{\pi}{4} [d_o^2 - d_i^2] = 1590\text{mm}^2$$

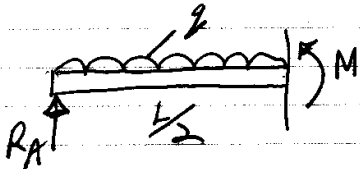
$$J = \frac{\pi}{32} [d_o^4 - d_i^4] = 1.834 \cdot 10^6\text{mm}^4$$

$$I = \frac{J}{2} = 0.917 \cdot 10^6\text{mm}^4$$

$$\text{Mass} = \rho AL = 7800 \cdot 1590 \cdot 10^{-6} \cdot 2 = \underline{\underline{24.8\text{kg}}}$$

$$\therefore \text{UDL } q = \frac{24.8 \cdot 9.81}{2} = \underline{\underline{121.6\text{ N/m}}}$$

$$R_A = R_B = \frac{qL}{2} = \underline{\underline{121.6\text{ N}}} \quad [12\text{ marks}]$$



$$M - R_A \cdot \frac{L}{2} + q \cdot \frac{L}{2} \cdot \frac{L}{4} = 0$$

$$M = R_A \frac{L}{2} - \frac{qL^2}{8} = 121.6 - 60.8$$

$$= \underline{\underline{60.8\text{ Nm}}} \quad [4\text{ marks}]$$

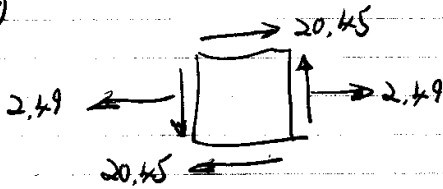
Bending Stress  $\sigma = \frac{My}{I} = \frac{60.8 \cdot \frac{75 \cdot 10^{-3}}{2}}{0.917 \cdot 10^{-6}} = \underline{\underline{2.49\text{ MPa}}} \quad [4\text{ marks}]$

Torsional Shear Stress  $\tau = \frac{Tr}{J} = \frac{1000 \cdot \frac{75 \cdot 10^{-3}}{2}}{1.834 \cdot 10^{-6}} = \underline{\underline{20.45\text{ MPa}}} \quad [4\text{ marks}]$

(ii)

MM2MS2 2011-12

Q1 (cont)



Mohr's Circle

$$C = \frac{\sigma_x + \sigma_y}{2} = \frac{2.49}{2} = 1.245 \text{ MPa}$$

$$R = \sqrt{\left(\frac{\sigma_x - \sigma_y}{2}\right)^2 + \tau^2} = \sqrt{\left(\frac{2.49}{2}\right)^2 + 20.45^2} = 20.49 \text{ MPa}$$

Principal Stresses

$$\sigma_1 = C + R = 21.74 \text{ MPa}$$

$$\sigma_2 = C - R = -19.25 \text{ MPa}$$

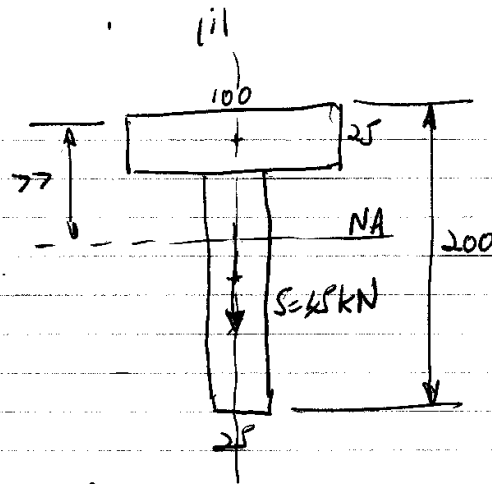
Max Shear Stress

$$\tau_{\text{max}} = R = 20.49 \text{ MPa}$$

[9 marks]

Q2

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(a) 2nd Moment of Area

$$\begin{aligned}
 I_{NA} &= \frac{bd^3}{12} + Ah^2 = \frac{100 \cdot 25^3}{12} + 1000 \cdot 25 \cdot (77 - 12.5)^2 \\
 &\quad + \frac{25 \cdot 175^3}{12} + 25 \cdot 175 \cdot (112.5 - 77)^2 \\
 &= 130,208 + \cancel{11.165} \cdot 10^6 \\
 &\quad + 11.165 \cdot 10^6 + \cancel{5.514} \cdot 10^6 \\
 &= \underline{2.721 \cdot 10^7 \text{ mm}^4}
 \end{aligned}$$

[10 marks]

(b) Flange-web join

$$\begin{aligned}
 \sigma &= \frac{SA\bar{y}}{I_z} = \frac{45 \cdot 10^3 \cdot 100 \cdot 25 \cdot [77 - 12.5]}{2.72 \cdot 10^7 \cdot 100} \\
 &= \underline{2.67 \text{ MPa}} \text{ above join}
 \end{aligned}$$

$$\sigma = 2.67 \cdot \frac{100}{25} = \underline{10.68 \text{ MPa}} \text{ below join}$$

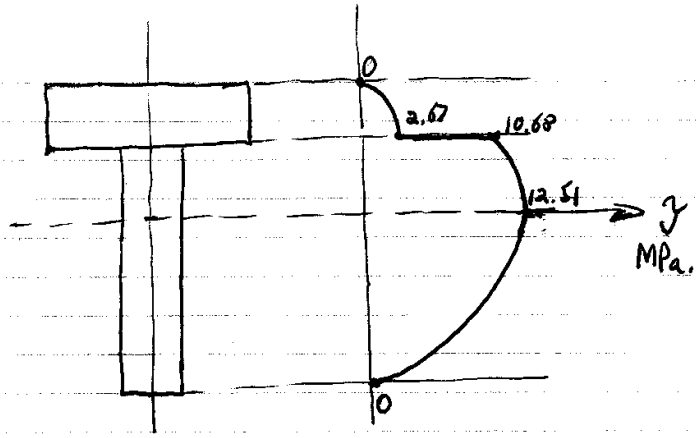
Neutral Axis

$$\sigma = \frac{SA\bar{y}}{I_z} = \frac{45 \cdot 10^3 \cdot \cancel{125} \cdot 25 \cdot \cancel{61.5}}{2.72 \cdot 10^7 \cdot 25} = \underline{12.51 \text{ MPa}}$$

[18 marks]

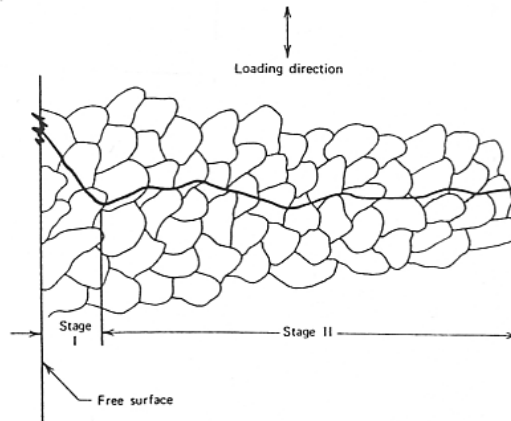
Q2 (cont)

Sketch



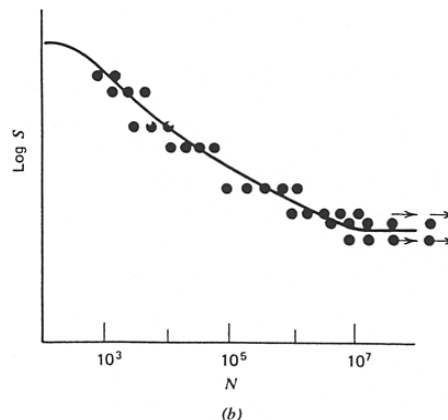
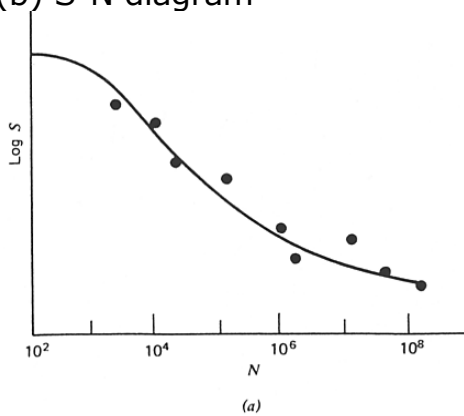
[5 marks]

- Q3 (a) Crack initiation, crack propagation and final fracture, as follows:
- (i) Stage I crack growth: development of persistent slip bands resulting from dislocations moving along crystallographic planes leading to stress concentrations
  - (ii) Stage II crack growth: the fatigue cracks tend to coalesce and grow along planes of maximum tensile stress range.
  - (iii) Final fracture; this occurs when the crack reaches a critical length and results in either ductile tearing (plastic collapse) or cleavage (brittle fracture)



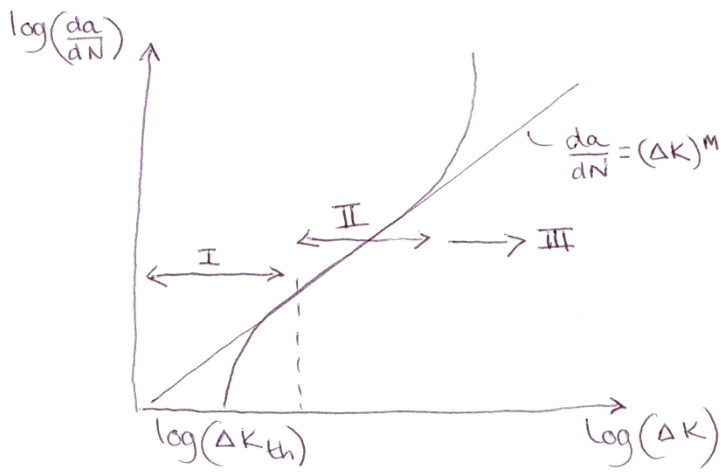
[10 marks]

(b) S-N diagram



Paris law shows there is a logarithmic linear relationship between crack growth rate and the stress intensity factor range during cyclic loading of cracked components

$$\frac{da}{dN} = C (\Delta K)^m$$



[10 marks]

(c)

At fracture

$$K_{IC} = 1.12 \sigma (\pi a_{crit})^{1/2}$$

$a_{crit}$  = critical crack length

Therefore

$$a_{crit} = (1/\pi) \times (K_{IC} / 1.12\sigma)^2$$

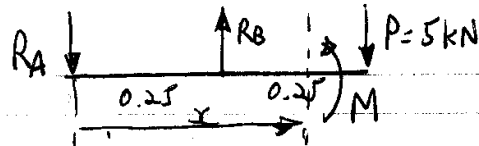
$$a_{crit} = (1/\pi) \times (200 / 1.12 \cdot 150)^2 = 0.451 \text{ m}$$

[13 marks]

(i)

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Q4. (a)



$$\downarrow \quad R_A + 5000 = R_B$$

$$EI = 10^4 \text{ Nm}^2$$

$$\circlearrowleft \quad R_A \cdot 0.25 = 5000 \cdot 0.25$$

$$R_A = \underline{5000 \text{ N}}$$

$$R_B = \underline{10,000 \text{ N}}$$

[4 marks]

Moments  $M + R_A x - R_B(x - 0.25) = 0$

$$EI \frac{d^2 y}{dx^2} = -M = R_A x - R_B(x - 0.25)$$

$$EI \frac{dy}{dx} = \frac{R_A x^2}{2} - \frac{R_B(x - 0.25)^2}{2} + A$$

$$EI y = \frac{R_A x^3}{6} - \frac{R_B(x - 0.25)^3}{6} + Ax + B$$

[8 marks]

Boundary Conditions

when  $x=0$   $y=0$   $\therefore B=0$

when  $x=0.25$   $y=0$   $\therefore 0 = \frac{R_A(0.25)^3}{6} + A \cdot 0.25$

$$\therefore A = -\frac{R_A(0.25)^2}{6} = -\underline{52.08} \text{ [marks]}$$

(ii) when  $x=0.5$   $EI y = \frac{5000(0.5)^3}{6} - \frac{10,000(0.25)^3}{6} - 52.08 \cdot 0.5$

$$= 10 \times 2 - 26.04 - 26.04$$

$$= \underline{52.12}$$

$$\therefore y = 52.12 \cdot 10^{-4} = \underline{5.21 \text{ mm}} \text{ [marks]}$$

(ii)

MM2MS2 2011-12

Q1(a)(cont)

$$(ii) \text{ when } x=0.5 \quad EI \frac{dy}{dx} = 5000 \cdot \frac{0.5^2}{2} - 10,000 \frac{(0.25)^2}{2} - 52.08$$

$$= 625 - 312.5 - 52.08$$

$$= \underline{260.42}$$

$$\frac{dy}{dx} = 260.42 \cdot 10^{-4} = 0.026 \text{ radians}$$

$$= \underline{1.5^\circ} \quad [\text{marks}]$$

(b) At B  $R_B = 10,000 \text{ N}$   
 $k = 20 \cdot 10^6 \text{ N/m}$

$$\therefore \delta_B = \frac{10,000}{20 \cdot 10^6} = \underline{0.5 \text{ mm}}$$

Boundary condition

when  $x=0.25$   $y=0.5$

$$\therefore 0.5 \cdot 10^{-3} \cdot EI = \frac{5000 \cdot 0.25^3}{6} + 10,000 \cdot A \cdot 0.25$$

$$\therefore A \cdot 0.25 = 0.5 \cdot 10^{-3} \cdot 10^4 - \frac{5000 \cdot 0.25^3}{6}$$

$$= 5 - 13.02 = \underline{-8.02}$$

$$\therefore A = \underline{\underline{-32.08}}$$

$$\therefore \text{ when } x=0.5 \quad EI y = \frac{5000 \cdot (0.5)^3}{6} - \frac{10,000 \cdot (0.25)^3}{6} - 32.08 \cdot 0.5$$

$$EI y = 104.17 - 26.04 - 16.04$$

$$= \underline{62.09}$$

$$\therefore y = 62.09 \cdot 10^{-4} = \underline{\underline{6.21 \text{ mm}}}$$

[9 marks]



⑤ Hoop & axial stresses due to pressure of  $p$ :

$$\sigma_{\theta} = \frac{pR}{t} = 40p$$

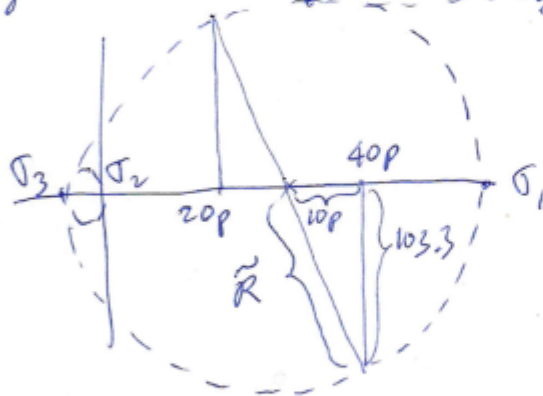
$$\sigma_z = \frac{pR}{2t} = 20p$$

shear stress due to the torque of  $1 \text{ kNm}$ :

$$\tau = \frac{TR}{I_p} \text{ where } I_p = \frac{\pi}{32} (D_o^4 - D_i^4) = \frac{\pi}{32} (80^4 - 78^4) = 387294 \text{ mm}^4$$

$$= \frac{1 \text{ kNm} \times 40 \text{ mm}}{387294 \text{ mm}^4} = 103.3 \text{ MPa}$$

(i) Using Tresca criterion:  $\tilde{\sigma}_y = 125 \text{ MPa}$



$$\tilde{R} = \sqrt{(10p)^2 + (103.3)^2} \leq \tilde{\sigma}_y$$

$$100p^2 + 103.3^2 \leq 125^2$$

$$p^2 \leq \frac{125^2 - 103.3^2}{100}$$

$$\therefore p_{\max} = \sqrt{\frac{125^2 - 103.3^2}{100}} \text{ MPa} = \underline{\underline{7 \text{ MPa}}}$$

(ii) Using von Mises criterion:

The principle stresses are:

$$\sigma_1 = 30p + \tilde{R} \quad ; \quad \text{where } \tilde{R} = \sqrt{(10p)^2 + (103.3)^2}$$

$$\sigma_2 = 0$$

$$\sigma_3 = 30p - \tilde{R}$$

$$(\sigma_1 - \sigma_2)^2 + (\sigma_2 - \sigma_3)^2 + (\sigma_1 - \sigma_3)^2 \leq 2\sigma_y^2$$

$$(30p + \tilde{R})^2 + (30p - \tilde{R})^2 + \tilde{R}^2 \leq 2\sigma_y^2$$

$$2(900p^2 + \tilde{R}^2) + \tilde{R}^2 \leq 2\sigma_y^2$$

$$1800p^2 + 3\tilde{R}^2 \leq 2\sigma_y^2$$

$$1800p^2 + 3(100p^2 + 103.3^2) \leq 2 \times 250^2$$

$$2100p^2 + 3 \times 103.3^2 \leq 2 \times 250^2$$

$$p^2 \leq \frac{2 \times 250^2 - 3 \times 103.3^2}{2100}$$

$$\therefore p_{\max} = \underline{\underline{6.65 \text{ MPa}}}$$